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DAMS AND
DAMMING,

A
THESIS

For

THE DEGREE

of

C.E.

Dams and Damming

A Thesis for the Degree of Civil Engineer

by

Jay Cullings.

Dams in general may be divided into the three distinctive titles of Cofferdams, Weirs, and Reservoir dams.

Cofferdams are devices for the exclusion of water from areas upon which are to be constructed foundations for buildings.

The most simple as well as the most usual form consists of two rows of piles driven around the area to be enclosed, one within the other and about four feet apart, this distance is regulated by the depth of water and strength of frame. The piles are connected by stringers or wall pieces which are notched and bolted to the piles. The string pieces of the inner row are placed on the sides of the piles toward the side to be enclosed and those on the outer toward the water. These stringers are placed about two feet above the level of the water.

Cross pieces of square timber connect the two rows of piles, upon which they are notched and bolted to the stringers.

These serve both to prevent the piles from spreading and as floor beams.

for the floor that supports the machinery used in construction

On opposite sides to the wall pieces are placed interior stringer pieces which serve as guides and supports to the sheeting piles

When the water is more than two ft deep there should be other stringer pieces both to the sides & across near to the top in bottom to give additional strength to the sheeting or to give the dam a water strength by a frame work which occupies the enclosed area. Some times it becomes necessary to resort to both measures.

Against the interior stringer, serving as guides are drawn the sheeting piles. Rivets are spiked into the interior of the sheeting piles, through them to the inner tie.

A puddling of clay is now rammed in between the sheeting, and the water pumped from the enclosed area.

The stability of this structure depends upon the space filled with puddling between the two rows of sheeting piles, the specific gravity of the puddling and the strength of the frame.

It is required to find the stability of a copper in a depth of water of 10 ft. The depth of puddling 12 ft. Width between rows of sheeting 4 ft.

The moment of the puddling, held to form by the frame, about the crown

inner edge, comprising a unit's width,
is $120 \times 12 \times 4 \times 2 = 1152$ Wt of a cu ft of clay =
120 lbs

The wt. of the water into its corner
of $\frac{1}{3}$ the depth of water is

$$0.25 \times 10 \times 10 \times \frac{1}{2} \times 3 \frac{1}{2} = 1041.6$$

Hence the clay held to place by the frame
would be stable without the additional
strength of frame, since the moment of
the clay exceeds that of the water by
104 lbs and will only require the frame to
hold it firmly in place. Then if we find
the dimensions of the piece required to
perform this duty, the stability of the
dam is determined.

The weight to be supported by each in-
ner foot of the dam is given by
taking the weight of a prism of clay the upper
side of which is of which is horizontal the
side toward the interior of dam is inclined
to the vertical at an angle of $42.4^\circ = \frac{1}{2}$ the
angle of repose of clay = 80° and the other
which coincides with the pile, is vertical
and equal to 12 ft.

$\log \tan 8^\circ + \log 12 + \log 2 = 1.075205 = \log$ weight
of the prism and

$1.075205 - \log 120 = 3.084810 = \log 124.27 =$ The
wt of prism to be supported

A pile in this frame is in the condi-
tion of a beam fixed at one end, supported
at the other and loaded at the point of
support. The load is transmitted

To the stringer by the upper ends of the
sheeting piles and then to the cross piece
which have to sustain the total weight
by their shearing strength of the notches.

We know then to find the weight trans-
mitted by the sheeting piles to the cross
pieces. To do this we consider a unit width
of the sheeting as a beam fixed at one end,
supported at the other and loaded at $\frac{1}{2}$ its dimen-
sion from the top to find the strain
upon the notch to which is the top of the
sheeting pile, and then this stress by R_1

$R_1 + R_2 = 1214$. For being that at the bottom
and $\frac{R_1}{R_2} = \frac{8}{7} \therefore R_1 = 2 R_2 = 818$ lb pressure to
the summer foot on the stringer

Now if we place the cross piece at 7
apart each notch must have longitudinal
shearing strength of 3232 lb

If heavy machinery is to be used
in construction and other loads are to be
supported upon the top of the dam the
piles will act as pillars, and cross pieces
as beams and must be made large
enough to support the maximum load
as well as to give additional strength
to the dam.

1 Weir. When a dam discharges the waste
2 water over its top it is called a weir.

3 In many small dams the water
4 flows over the whole length of the dam
5 at certain times and the whole must
6 be built to withstand the pressure and
7 the action of water in falling over.

8 Many devices are resorted to for getting
9 a head of water in shallow streams, such
10 as a dam formed by simply piling in
11 boulders as well as other rude devices, but
12 more often a systematic and economical
13 method is adopted as log dams, frame
14 dams, stone dams etc.

15 Log dams are made use of in sit-
16 uations where the head of water required
17 is small, stream is narrow and depth of
18 water not great.

19 In situations where timber is plenty
20 a log dam is made with the least
21 expenditure of money and labour.

22 A very economical form is that in
23 which the trees are felled and the
24 trunks lopped then placed in a layer
25 on the bed of the stream, with the tops
26 up stream, and circumvented in between
27 them. After this has been done
28 another should be started laying the
29 butts several feet back of the first
30 and more clay put in than back
31 of these the dam proper is begun
32 and laid in tiers with joints of

timber placed between clay filled
 in between. These tiers should be
 laid so that the plan of the butte
 shall be in the form of an arc
 of a circle. The first layers with their
 projecting butte form protection
 to the foundation of the dam, from
 the eroding action of the water.

In each bank is built a crib filled
 with clay to protect the side of the
 dam.

It is required to build a dam across
 a stream in which the greatest prob-
 able depth of water, after the erection
 of the dam is 10 ft. Height of dam is
 6 ft.

The length of logs at the bed joint is
 given by the following formulae

$$z^2 = \frac{w'k^3 \sec^2 j}{6\pi(q-q')w'h} - 2 \frac{wk^2(\frac{q}{2} + \frac{1}{4}) \tan j}{2\pi(q-q')w'h} z$$

In which z = the length of logs, w' = weight
 of 1 cu ft of water, j the angle that the
 inner face makes with the vertical
 $\therefore \sec j = \frac{z}{h}$ and $\tan j = \frac{h}{\sqrt{z^2 - h^2}}$ in which h is
 the height of the dam, x the depth of
 the joint below the surface of the
 water, q = the ratio of the centre of resis-
 tance from the middle point of
 z which may be taken as $\frac{1}{4}$, q' = the ratio
 of the distance of the point where a
 line drawn vertically from the centre
 of gravity would cut the line z , given

The middle point of F to the whole length t , which is $\frac{1}{6}$, so is the weight of 1 cu ft of the material used in construction and n a numerical factor depending upon the figure of section which for a triangle is $\frac{1}{2}$ making the substitution in the above formulae then obtains the following result.

$$F^2 = \frac{62.5 \times 1000 \times \frac{F^2}{36}}{6 \times \frac{1}{2} (\frac{1}{4} - \frac{1}{6}) 70 \times 6} = \frac{62.5 \times 100 (\frac{1}{2} + \frac{1}{4}) \frac{6}{F^2} = 107 F}{\frac{1}{2} (\frac{1}{4} - \frac{1}{6}) 70 \times 6} F$$

whence $F = 12.5 F$

In building a dam of this sort logs of much greater length are usually at hand and may as well be used their full length as to be cut off. The logs in each layer being of the same length a slight batter is given to the down stream face.

If the above conditions are complied with the dam will be stable at all its joints, provided the one at the bed joint is stable and for it to be stable the following condition must be complied with

$$\frac{W'}{L} + W \tan j \leq \tan \phi$$

In which ϕ = the angle of repose of the material at the foundation which for clay is about $18\frac{1}{4}^\circ$ and the $\tan \phi$ about $.33\frac{1}{3}$ making the substitution in the preceding formulae

$$\frac{62.5}{70 \frac{12}{10} + 62.5 \times 9.0496} = .44 \text{ which is greater than}$$

The tan of angle we must increase the length the length, suppose we increase the length of foundation by means of projecting tiers to 40 ft for the value of the equation about .19 which would give the entire structure stability on a bed of clay.

Frame Dams, The application of frame dams is coming into quite general use in river improvements and this application has produced a form of dam peculiarly adapted to its use, This class of dams will be considered another one.

An example of a simple frame dam of wood is the following. It consists of a series of triangular frames resting upon sills lying across the stream in a direction perpendicular to its axis, and which rest upon short blocks of wood.

These frames are covered by means of ties or floor beams which support a sheeting of boards. To prevent leakage these may either be tongue and groove or battened and then pitched or any other common method which is convenient.

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The frames consist of a beam
or sill laid from one of the lower
sills across to the other in a direct-
ion parallel to the axis of the stream,
an inclined beam which is supported
at the up stream end by the sill
at the down stream end by an
inclined strut which runs up
from the down stream end of the
sill perpendicular to the end of the
inclined beam, and at the center
of pressure of the water by another
inclined strut perpendicular to the
beam.

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The dam is anchored to place by the
down stream sill projecting into a crib
of logs in either bank and filled
with clay and stones and the up-
stream sill resting against the
side of the crib, also by iron rods
which pass through sills of frame
and down into holes drilled into rocks
in the bed of the stream secured by
having the lower end split and an iron
wedge started in the cleft, which when
the rod is driven home spreads the
rod to fit the drill hole.

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That this dam should have the strongest
form and to distribute the pressure
as more equally it should be built in, then,
the shape of a bottom of a hollowed
convex or concave, in this form the

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pills would be chords of a circle cut
from an arch having for their abutments
the exits in the banks.

It is required to find the dimensions
of the pieces of a dam, where the
maximum depth of water shall not
exceed 12 ft, height of dam 8 ft, length
of oblique beam 26 ft.

To find the stress upon the oblique
beam, and consequent dimensions, we
now to consider a beam with three
supports one at each end and one at an
intermediate point, the action of pressure
of the water, under an uniform load
and a constant increasing load from
the top downward.

Let a = the depth of the upper end of
the beam below the surface of the water (4 ft),
 b = height of dam (8 ft), c = the length of
the beam, 26' the portion of the beam
between the upper point of support
and the one at center of pressure, 22'
between bottom and center of pressure
 Q' the point of support at the top, Q
support at bottom and, P support a center
of pressure.

Letting w = the intensity of the uni-
formly increasing load, and w' of the
intensity of the uniformly distrib-
uted load, we shall have for the
lengths of the pieces between
the ends and the center of pres-

we.

$$2Z' = \frac{\int_0^{20} (a + Zk) w dx}{\int_0^{20} (a + Zk) w dx} = \frac{2c}{3} \frac{a + 2b}{a + b} = 10^{2/3}$$

$$\therefore 2Z = 9^{1/3}$$

We now have given $2Z' = 10^{2/3}$ $2Z = 9^{1/3}$ $2c = 20$, $a = 4$, $b = 8$ and $w' = 62^{1/2}$ to find the weight upon Q' , Q , and P .

$$Z^2 Q + Z'^2 Q' = \frac{2w'}{8} (2dZ'^3 - 4/5 Z'^4) + \frac{2w'}{8} (2dZ^3 - 4/5 Z^4) \quad (1)$$

$$- Z^2 Q + Z'Z Q' = 1/3 Z Z' (a' + 2a) w' - 1/3 Z^3 (a' + 2b) w' \quad (2)$$

$$Z'^2 Q' + Z'Z Q' = \frac{2w'}{8} (2dZ'^3 - 4/5 Z'^4) + \frac{2w'}{8} (2dZ^3 - 4/5 Z^4) \quad (3)$$

$$+ 1/3 Z Z' (a' + 2a) w' - 1/3 Z^3 (a' + 2b) w'$$

$$2cZ = b - a$$

$$a' = a + 2Z'Z$$

$$Zd' = a' - 2a$$

$$dZ = a + 2b$$

$$\log Z' = \log(10^{2/3}) = .726949$$

$$" Z = " \frac{25}{6} = .669087$$

$$" 1/3 = 9.622876$$

$$" a' = \frac{9Z}{15} = .787697$$

$$" a = " 4 = .602060$$

$$" b = " 8 = .403040$$

$$" d' = " \left(\frac{212}{8}\right) = 1.150245$$

$$" d = " \frac{42}{3} = 1.141628$$

$$" 4/5 = 9.602660$$

$$" Z^2 Q' = 1.453998 + \log Q' = \log(27.44 Q')$$

$$" Z Z' Q' = 1.396086 + " Q' = " (24.889 Q')$$

$$" 1/3 Z Z' a' w' = 1.706079 + " w' = " (60.884 w')$$

$$" 2/3 Z Z' a w' = 1.821972 + " w' = " (66.39 w')$$

$$" 1/3 Z^3 a' w' = 2.317694 + " w' = " (207.77 w')$$

$$" 2/3 Z^3 b w' = 2.735017 + " w' = " (135.5 w')$$

$$\log \frac{Zw'}{4} d' z^3 = 3.821892 = \log 6623.6$$

$$\log \frac{Zw'}{10} l^4 = 2.603876 = \log 401.68$$

$$\log \frac{Zw'}{4} d z^3 = 3.648999 = \log 4456.6$$

$$\log \frac{Zw'}{10} l^4 = 2.398724 = \log 250.46$$

Substituting these values in the preceding formulae we shall have.

$$0.2.32 Q' = (6623.6 - 401.68) + (4456.6 - 250.46) \\ - (129856.25 + 8468.76) = 119943$$

$$Q' = -22937$$

Now if the frames are placed 9 ft apart, Q' becomes 18349* which substituted in eq (1) gives $Q = -15190$ * and these values in

$$P + Q + Q' = 7(a'+b)w' + 7'(a'+a)w' \quad (4)$$

$$P = -7007$$

Now if we make the beam uniform in cross section throughout its length as is usual, take moments at P we shall have the dimensions given by

$$18349 \times 10^{2/3} = 4,10000 h^2$$

$$140722 = 4,10000 b h^2$$

$$b h^2 = 1174 \frac{1}{3}$$

It will have to make the beam about 10" x 12"

The struts are given by the following formulae. For strut down stream

$$1) \quad 18349 = \frac{10000}{1 + \frac{2}{125} \frac{l^2}{h^2}} \quad \text{in which } l = \text{section in eq in.}$$

h = dimension in the direction of least resistance, l = length in inches,

solving (1) we have the strut about $8" \times 10"$ also for the strut below the centre of pressure.

$70093 = 1000bh \therefore bh = 70,093$ or $b=8$
 $h=9$ since the the pillar is short we may use 9×8 timber.

To find the dimensions of the floor beams or ties upon which the covering rests and the manner of distributing them.

Suppose we place the one at the bottom $1/2$ ft apart. Their dimensions are given as follows. We find the pressure of the water upon one which is $12 \times 8 \times 1/2 \times 62.5 = 9000$ which is divided equally between two beams.

Taking moments about one end
 $4500 \times 4 = 18000$ hence the dimensions are given by

$$18000 = 1/6 \cdot 1000bh^2$$

\therefore Floor beams are $3" \times 6$

Now the load increases directly as the distance from the top, hence the distance between them may increase directly as distance from the bottom.

We find that the distance between them at the top may be $4 \frac{1}{2}$ ft.

Then there should be 7 pieces 3×6 each placed $3 \frac{1}{2}$ ft. higher up the frame than that preceding it.

Improvements of rivers for slack water navigation have produced a peculiar class of dams which are admirably adapted to the purpose which they serve.

Many rivers are made navigable by damming and dikes forming long pools deep enough for the draught of the boats to be used in their navigation.

The boats are passed from one pool to another by means of locks. This system is known by the title of locks and dams.

Fixed dams have been used for this purpose, but there is a great objection to these, in that, when the water in the stream is high enough for navigation without the aid of the lock and dam, that they are in the way of the free passage of boats up and down the stream.

To overcome this difficulty, movable dams have been introduced. There are two classes of these movable dams in general use.

(1) Those which are manouvered by the aid of machinery and

(2) Those which are operated by the water itself.

Of the first class are the Pierre Point and Noble dams, The Chauvine

wicket dam and the Bouli dam of
trestles and sliding gates.

Of the second are the Bear trap
dam, Desfontains and the Grand
dam.

The Pivou dam consists of a
series of trapezoidal trestles of iron,
placed across the river parallel to
its axis and about four feet apart.

These are securely fastened to the
bottom by means of an anchorage
at each end of the lower portion,
which is called the axis, and has at
each end a journal which enters
a journal box, which is anchored
to the foundation.

The tops of the trestles are secured
by connecting bars. The bars on the
up-stream side are made stronger
in order to support the upper ends
of the needles. The up-stream mem-
ber is called the stilt and stands
almost vertical, the down stream
one is more inclined and extends
from the axis up to a level with
the top of the up stream member,
and is connected with it by means
of a cap. A diagonal member con-
nects the top of the up stream
member with the bottom of the
down stream member.

The trestles revolve about the axis

1 in a direction perpendicular to the
2 axis of the stream.

3 The needles are square wooden
4 bars, which may be as much as fif-
5 teen feet long. They rest against
6 the connecting bar at top and against
7 a sill set in the foundation at the
bottom

8 There are two manœuvres of a
9 needle dam, closing and opening.

10 The method of closing is as follows
11 Suppose the dam to be entirely open
12 with the trusses lying down. The work
13 ers begin by grappling for the truss
14 at the end of the pier which is
15 upper-most, with a long handled hook

16 The truss may be raised by hand if
17 not too heavy, but if it is the hook
18 is fastened to a rope, by means of
19 a ring in the end of the handle, and
20 raised by a windlass, when the truss
21 is nearly up a man grapples it
22 with a hand bar, with two clutches
23 just the distance apart of two trusses
24 when up. The truss is held up
25 by this bar until the second is raised
26 when the connecting bar and plank
27 are put in place, and the others raised
28 in a similar manner.

29 When all of the trusses are raised to
30 place and secured and secured the needles
31 are put in place, one by one, from
32

1 from the top of the dam.

2 The objections to this dam are the
3 number of pieces to be handled, and that
4 there frequently occur sudden rises
5 in the water of rivers so that a dam
6 of this description cannot be let
7 down and thus form the same ob-
8 struction as a fixed dam.

9 The last objection has been overcome
10 in the Belgian Meuse dam by letting
11 the needles between two trestles all
12 go at once by means of a jacking post
13 which is enclosed in an upright tube
14 which extends up from the top of the
15 up-stream member and acts as a post
16 for the support of the bridge which
17 is made of shut iron, in sections
18 of the width of the bridge and which
19 are supported at one end on a trestle
20 and at the other hinged to a bar on
21 the top of the other trestle. The jacking
22 post is movable within the tube
23 and is cut half way through by a notch
24 near the bow end, about three inches
25 long. There is also a corresponding
26 notch in the tube. The jacking post
27 projects up above the bridge, ter-
28 minating in a square end for
29 the application of a wrench.
30 The needles are supported by a
31 bar, one end of whose end is hinged
32 to a trestle and the other supported

within the slot in the tube containing the jack post turned so that the slot in it is away from the one in the tube, and when turned by a wrench at top brings it around to correspond with that in the tube, thus leaving the bar without support and it swings around to the side of the truss to which it is attached. The nuts are prevented from being lost by a rope which passes through the handle and is fastened to the bridge.

The Chanovic Dam consists of a series of panels independent of each other, three or four feet wide, and separated from those along side by an interval of about four inches. The panels are of wood and rectangular in shape, they oscillate about the top of a frame called a horse, which revolves about an axle at the bottom in a direction parallel to the axis of the stream and is capable of swinging its top down to the foundation in a down stream direction but is checked in the upstream direction by a sill fixed in the foundation. The top of the horse is held to place by a prop, the upper end being attached to the head of the

1 horse, and the bottom rests against a
2 shoulder or hutter. A panel, horse ^{prop}
3 prop as uprise a wicket.

4 The axle of oscillation of the panel
5 is placed at one half the length of
6 the panel, in the pass, ^{and} those of the
7 wickets at a little greater distance above
8 the centre of pressure in.

9 When the water rises so as to raise
10 the centre of pressure above the point
11 of support the panel is swung.

12 The wickets of the pass should
13 never swing. The face of the panel makes
14 an angle with the vertical of 20°
15 and the top inclined away from the water.

16 The horse and the prop incline toward the
17 water, at angles of 5° and 53° respectively.

18 The wicket is up when the prop
19 rests against the hutter, and the
20 lower end of the panel rests against
21 the sill. It is swung when the foot
22 of the panel is up so that the
23 water passes above and below it, ^{and}
24 low when lying down upon the floor.

25 The Bouli dam is essentially the
26 same as the Poiri except that in the
27 places of needles are substituted sliding
28 panels made of a number of planks
29 laid horizontally and manouvered by a
30 windlass. This form is not in general use.

31 The Bear Trap dam consists of two
32 leaves having their axes of revolution

1 in the floor. One is arranged so
2 as to fall up and the other down-
3 stream. The one that falls up-stream
4 has its axis downstream and the
5 other visa versa

6 When the dam is down the upper
7 leaf projects over the lower. A bar
8 of wood is placed near the top of
9 the upper gate and fastened to the
10 lower side so that when they are
11 raised together the two gates shall
12 check each others motion by the
13 upper edge of the lower coming
14 in contact with the timber on the
15 upper.

16 An auxiliary gate is placed
17 in front of the up-stream gate
18 to aid in raising the dam by form-
19 ing a temporary head of water.

20 A channel runs through the
21 pier for the passage of water from
22 the upper pool to the lower with
23 a side passage leading under the
24 gates. Each end is provided with
25 valves.

26 When the dam is to be raised the
27 lower valve is closed and the upper
28 opened. The auxiliary, which is
29 secured to the bed by an apron immed-
30 ially in front of that of the upper
31 gate. The upper end of this gate is
32 held down to the bottom by a
33

1 trigger, the trigger tripped by a trig-
2 ger rod which extends up from the
4 bottom to the surface of the water.
4 When the gate is loosened by the
5 rod the end rises by its own buoyancy
6 until it is caught by the current
7 and raised to position thus forming
8 a temporary dam. The water raising in
9 the upper pool exerts a pressure
10 upon the gates and raises them to position.
11 The gates remain up as long as
12 the water is kept in the channel.
13 To let the water out the upper
14 valves are closed and lower ones opened.

Reservoir walls are built for the purpose of storing up water for the use of cities, canals, manufactures & agriculture and to prevent the inundation of adjacent lands.

Frequently dams are constructed so as to serve several of these duties by the creation of only one structure.

One of the most noteworthy of the modern dams is that on the river Furroul near the town of St Etienne.

The conditions imposed upon this dam was that the reservoir should supply the town and factories with water and prevent the town from inundation.

To accomplish the first of these duties a subterranean channel was cut through the counterfort against which the wall rests. The reservoir end of which is stopped with masonry through this masonry pass two cast iron pipes 4 in in diameter, which are supplied with stop cocks and which carry the water to a bay from which the reserved water is conducted through ~~an~~ open channel to the bed of the river, regulated by a regulator sluice. Though a covered channel also provided with a reg-

ulator since, from which it
may ^{be} either by means of a pipe
directly or by means of a small
reservoir, conducted to the St
Etienne aqueduct

The second purpose which it
was to serve was accomplished
by gates at the head of the reser-
voir. One of which shuts off the water of the
river from the reservoir and the other that of the
waste. Just above the gates there is
a scale which shows the depth of
the water. When the water reaches the
depth of 93 cm in the second the town
begins to be inundated. The reservoir
is not allowed to fill to a depth ex-
ceeding 44.5 ^m when the discharge is
below 92 cm in height. This is done
by closing the gate between the river
and reservoir and opening that of
the waste.